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## An implicit material point method applied to granular flows

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### Abstract

The main objective of this work lies in the development of a variational implicit Material Point Method (MPM), implemented in the open source Kratos Multiphysics framework. The ability of the MPM technique to solve large displacement and large deformation problems is widely recognised and its use ranges over many problems in industrial and civil engineering. In the current work the continuum based implicit MPM is applied to engineering applications, where granular material flow is involved.

For the resolution of the length and time scale of these particular problems, both continuum and discrete models are typically used. Even if discrete techniques predict more feasible results, nowadays, their use is limited to the investigation of element tests of particles, or to the simulation of reduced systems, not allowing to make important decisions in the analysis and design of granular processes. Some advantages of MPM over discrete methods are tested, such as, the ability to simulate granular flow at the large scale with acceptable computational cost and the capability to get information of stress and strain state in a more straightforward way.

The focus of this paper is a comparative study between an irreducible and a mixed formulation, both implemented in the MPM code, to assess the improvement in accuracy and reliability of the numerical results when the latter formulation is adopted.

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### 1. Introduction

The numerical simulation of solid mechanics problems involving history dependent materials and large deformations, has historically represented one of the most important topics in computational mechanics.

Among the Lagrangian techniques, in the last decades, the Material Point Method (MPM) [1,2] has experienced an increasing popularity due to its capabilities of solving several complex engineering problems. This method has its origin in the particle-in-cell (PIC) method, formulated by Harlow [3] for the resolution of fluid flow problems. Some decades after, Sulsky presented its extension to solid mechanics [1,2]. The Material Point Method combines a Lagrangian description of the body of interest, which is represented by a set of particles, the so-called *material points*, or *MPM particles*, with the use of a *computational mesh* used to solve numerically the system of governing equations.

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At each time step, the governing equations are solved on the nodes of the computational grid, while history dependent variables and material information are saved on the particles for the entire deformation process.

With few notable exceptions [4–8], the majority of the MPM algorithms are written in an explicit formulation [9–11]. This approach is generally preferable when simulating impacts at high velocities, or fast transient problems. In other cases, for example when the driving force is the gravity, or when the rate of deformation is small, the adoption of an implicit time scheme is the best choice. In implicit scheme, in fact, the stability of the method (for properly chosen dissipative methods) does not depend on the wave propagation speed within the media, which provides the typical time step limitation for explicit approaches. It is worth mentioning the work of [8] where a comparison between an explicit and implicit time scheme is performed. In this work the advantages of using an implicit scheme are discussed, as for instance, the lower limitation on the time step size and the improvement of the algorithmic accuracy for elastoplastic constitutive law.

The MPM code employed in the current work has been developed by the authors within the Kratos Multiphysics open source platform [12,13]; the details of the formulation and the algorithm can be found in [14]. The code proposed is designed for an easy implementation of a wide range of pressure-dependent constitutive laws, chosen in function of the problem to be solved.

A preliminary study is performed to test the capability of an irreducible formulation (i.e., with displacement as only primary variable) and a mixed formulation (i.e., with displacement and pressure<sup>1</sup> as primary variables) to give reliable results when one wants to solve problems which involve granular flow under the assumption of large displacements and large deformations. In fact, it is well known that some issues might appear when an irreducible formulation is adopted [15]. For instance, when elastoplastic constitutive laws in the framework of J2-incompressible plasticity are considered, the numerical results can be affected by locking when the volumetric strain contribution is higher than the deviatoric one. The only way to guarantee a locking free behaviour for incompressible material and mesh independence, is to develop a mixed finite element, able to evaluate the results avoiding this drawback.

In the present work a granular column collapse is analysed, using an elastoplastic constitutive law with Drucker-Prager yield criterion. The authors demonstrate how using an irreducible formulation the numerical solution is affected by volumetric locking. Therefore, the adoption of a mixed formulation becomes fundamental; this latter formulation, which accounts for a second primary variable, commonly represented by the pressure, allows to correctly evaluate the strain field when the body experiences only an isochoric deformation and, as a consequence, to avoid some issues such as the volumetric locking.

## 2. Description of the problem

In this section the equations, which describe the physical problem under investigation, are presented. Firstly, the governing equations in strong form are illustrated. Secondly, the procedure to derive the linearised solving system of algebraic equations is briefly described and referenced. Finally, the Drucker-Prager plastic model is presented.

### 2.1. Governing equation in strong form

Let us consider the body  $\mathcal{B}$  which occupies a region  $\Omega$  of the three-dimensional Euclidean space  $\mathcal{E}$  with a regular boundary  $\partial\Omega$  in its reference configuration. A deformation of  $\mathcal{B}$  is defined by a one-to-one mapping

$$\varphi : \Omega \rightarrow \mathcal{E} \quad (1)$$

that maps each point  $\mathbf{p}$  of the body  $\mathcal{B}$  into a spatial point  $\mathbf{x}$

$$\mathbf{x} = \varphi(\mathbf{p}) \quad (2)$$

which represents the location of  $\mathbf{p}$  in the deformed configuration of  $\mathcal{B}$ . The region of  $\mathcal{E}$  occupied by  $\mathcal{B}$  in its deformed configuration is denoted as  $\varphi(\Omega)$ .

<sup>1</sup> In the present work the pressure variable refers to the volumetric stress field.

The problem is governed by mass and linear momentum balance equations

$$\frac{D\rho}{Dt} + \rho \nabla \cdot \mathbf{v} = 0 \quad \text{in } \varphi(\Omega) \quad (3a)$$

$$\rho \mathbf{a} - \nabla \cdot \boldsymbol{\sigma} = \rho \mathbf{b} \quad \text{in } \varphi(\Omega) \quad (3b)$$

where  $\rho$  is the mass density,  $\mathbf{a}$  is the acceleration,  $\mathbf{v}$  is the velocity,  $\boldsymbol{\sigma}$  is the symmetric Cauchy stress tensor and  $\mathbf{b}$  is the body force. Thermal effects are not considered in the present work, so the energy balance is considered implicitly fulfilled. The balance equations are solved numerically in a three-dimensional region  $\Omega \subseteq \mathcal{R}^3$ , in the time range  $t \in [0, T]$ , given the following boundary conditions on the Dirichlet ( $\varphi(\partial\Omega_D)$ ) and Neumann boundaries ( $\varphi(\partial\Omega_N)$ ), respectively

$$\mathbf{u} = \bar{\mathbf{u}} \quad \text{on } \varphi(\partial\Omega_D) \quad (4a)$$

$$\boldsymbol{\sigma} \cdot \mathbf{n} = \bar{\mathbf{t}} \quad \text{on } \varphi(\partial\Omega_N) \quad (4b)$$

where  $\mathbf{n}$  is the unit outward normal.

The constitutive model, which is needed to fully define the boundary value problem is described in detail in Section 2.3.

## 2.2. Weak form and linearisation of the weak form in spatial form

Following the standard FEM procedure, the weak form of the momentum balance equation is obtained by employing the Galerkin method. The  $L_2$  inner product of Equation 3b is derived using an arbitrary test function  $\mathbf{w}$ , such that  $\mathbf{w} = \{\mathbf{w} \in \mathcal{V} \mid \mathbf{w} = \mathbf{0} \text{ on } \varphi(\partial\Omega_D)\}$ , where  $\mathcal{V}$  is the space of virtual displacements. By using the divergence theorem the weak form of momentum balance can be obtained and expressed as

$$G(\mathbf{u}, \mathbf{w}) = \int_{\varphi(\Omega)} \boldsymbol{\sigma} : (\nabla^S \mathbf{w}) dv - \int_{\varphi(\Omega)} \rho (\mathbf{b} - \mathbf{a}) \cdot \mathbf{w} dv - \int_{\varphi(\partial\Omega_N)} \bar{\mathbf{t}} \cdot \mathbf{w} da = 0, \quad \forall \mathbf{w} \in \mathcal{V} \quad (5)$$

The previous equation has the same expression of a weak form under the assumption of infinitesimal strains and displacements. However, in this work material and geometric non-linearities are considered; thus, a linearisation of the weak form is needed. An expansion in Taylor's series of Equation 5, evaluated at the last known equilibrium configuration  $\mathbf{u}^*$ , is performed and expressed as

$$L(\delta\mathbf{u}, \mathbf{w}) \simeq G(\mathbf{u}^*, \mathbf{w}) + DG(\mathbf{u}^*, \mathbf{w})[\delta\mathbf{u}] = 0, \quad \forall \mathbf{w} \in \mathcal{V} \quad (6)$$

where  $L$  is the linearised virtual work function and  $DG(\mathbf{u}^*, \mathbf{w})[\delta\mathbf{u}]$  is the directional derivative of  $G$  at  $\mathbf{u}^*$  in the direction of  $\delta\mathbf{u}$ . The detailed procedure to derive the final expression of the solving system of linearised equations (Equation 6) in integral and discrete form can be found in [16] and [14].

Concerning the mixed formulation, linear finite elements formulated in a mixed displacement-pressure field are considered. In this regard, Equation 5 can be rewritten as

$$G(\mathbf{u}, \mathbf{w}) = \int_{\varphi(\Omega)} (\text{dev}(\boldsymbol{\sigma}) + p\mathbf{1}) : (\nabla^S \mathbf{w}) dv - \int_{\varphi(\Omega)} \rho (\mathbf{b} - \mathbf{a}) \cdot \mathbf{w} dv - \int_{\varphi(\partial\Omega_N)} \bar{\mathbf{t}} \cdot \mathbf{w} da = 0 \quad (7)$$

where the Cauchy stress tensor  $\boldsymbol{\sigma}$  is decomposed in its deviatoric  $\text{dev}(\boldsymbol{\sigma})$  and volumetric component  $p$ . The pressure field  $p$  in Equation 7 represents the second primary variable, determined by the volumetric part of the material model. As in the present work a Neo-Hookean material is adopted, the resultant continuity equation is given by

$$p - U'(J) = 0 \quad (8)$$

where  $U'(J)$  is the first derivative of the volumetric term of the free energy function  $\psi$ , which will be defined in the following section once the constitutive model is characterized. By performing a  $L_2$  inner product of Equation 8 with

an arbitrary test function  $q$ , such that  $q = \{q \in Q \mid q = 0 \text{ on } \varphi(\partial\Omega_D)\}$ , where  $Q$  is the space of virtual pressures, the weak form of the pressure constitutive equation can be obtained and expressed as

$$G(p, q) = \int_{\varphi(\Omega)} q [p - U'(J)] dv = 0, \quad \forall q \in Q \quad (9)$$

For the treatment of the incompressibility constraint a pressure stabilization is adopted. In this work the Polynomial Pressure Projection (PPP), introduced by Dohrmann and Bochev [17], is used. The determination of an additional term, which has to be added to Equation 9 to stabilize the mixed finite element, is explained in detail in [17] and [18].

### 2.3. Drucker-Prager law in finite strain plasticity

To complete the non-linear boundary valued problem a Drucker-Prager plastic model is considered. For the elastic response a hyperelastic Neo-Hookean model is considered, while the plastic behaviour is modelled following the theory of  $J_2$ -plasticity at finite strains developed by Simo and Hughes [19]. The algorithm adopted in the current work is the same one presented by Hofstetter and Taylor [20]. The present formulation uses a non-associative flow rule, assuming that the material is plastically incompressible, which means that the dilatation of the plastic strain is equal to zero. Under this assumption, an extension of the return mapping algorithm for  $J_2$  plasticity [19] is required.

As the Neo-Hookean material is represented by the following free energy function  $\psi$

$$\psi(\mathbf{b}) = U(J) + W(\mathbf{b}) = \frac{1}{2} K \ln^2(J) + \frac{1}{2} G \text{dev}(\bar{\mathbf{b}}) \quad (10)$$

written here as the sum of volumetric ( $U$ ) and deviatoric ( $W$ ) response, the Kirchhoff stress  $\boldsymbol{\tau}$  can be defined as

$$\boldsymbol{\tau} = K \ln(J) + \frac{1}{2} G \text{dev}(\bar{\mathbf{b}}) \quad (11)$$

where  $K$  and  $G$  are the bulk and shear moduli, respectively,  $J = \det(\mathbf{F})$  is the determinant of the total deformation gradient and  $\bar{\mathbf{b}}$  is the volume preserving part of left Cauchy-Green tensor, defined as

$$\bar{\mathbf{b}} = J^{-\frac{2}{3}} \mathbf{b} \quad (12)$$

Let  $\Psi$  denote a convex set of plastically admissible stresses

$$\Psi = \{\boldsymbol{\tau} : f(\boldsymbol{\tau}) \leq 0\} \quad (13)$$

the Drucker-Prager yield condition in terms of Kirchhoff stress is

$$f(\boldsymbol{\tau}) = |\text{dev}(\boldsymbol{\tau})| + \frac{\mu}{\sqrt{3}} \text{tr}(\boldsymbol{\tau}) - \sqrt{2}\kappa \quad (14)$$

where  $\mu = \frac{2\sqrt{2}\sin\phi}{(3+\sin\phi)}$  is a coefficient depending on the internal friction angle  $\phi$ , and  $\kappa$  the yield strength in simple shear. The constitutive relation, based on the decomposition of the total deformation gradient  $\mathbf{F} = \mathbf{F}^e \mathbf{F}^p$  into an elastic and a plastic part, denoted as  $\mathbf{F}^e$  and  $\mathbf{F}^p$ , respectively, can be written as

$$d\boldsymbol{\tau}_{n+1} = \widehat{\mathbb{C}}_{n+1} : (d\boldsymbol{\epsilon}_{n+1} - \boldsymbol{\epsilon}_{n+1}^p) \quad (15)$$

where

$$\boldsymbol{\epsilon}_{n+1}^p = \begin{cases} \lambda \frac{\partial G(\boldsymbol{\tau})}{\partial \boldsymbol{\tau}}, & \text{if } f(\boldsymbol{\tau}) = 0 \\ 0, & \text{if } f(\boldsymbol{\tau}) < 0 \end{cases} \quad (16)$$

where  $\widehat{\mathbb{C}}$  is the fourth order incremental constitutive tensor,  $\lambda$  is the plastic multiplier and  $G$  is the plastic potential, which in the present work can be expressed as  $G(\boldsymbol{\tau}) = |\text{dev}(\boldsymbol{\tau})|$ .

### 3. Numerical example

In the present work a preliminary study is performed to test the capability of an irreducible and a mixed formulation, which the implicit MPM code is based on, to obtain reliable results in problems where granular flow is involved under the assumption of large displacements and large deformations. In particular the authors are interested in demonstrating if the results are affected by volumetric locking, a consequence of the inability of the element to exactly represent the isochoric strain field, when an irreducible formulation is adopted and if the adoption of a mixed formulation can fix this issue.

In this section a planar granular column collapse test case, under the force gravity,  $\mathbf{g}$ , is considered. The geometry of the sample can be observed in Fig. 1, characterized by an aspect ratio  $a = \frac{h_0}{l_0} = 1.5$  and a bottom boundary where no-penetration and no-slip conditions are applied. The material and problem data used to run the simulations can be found in Table 1 and Table 2.

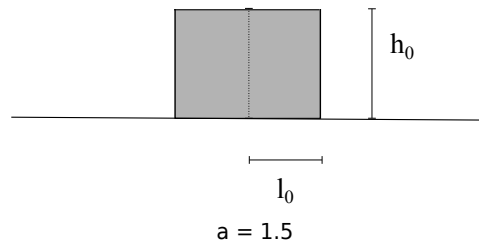


Fig. 1. Planar granular column collapse. Geometry.

Table 1. Planar granular column collapse. Material data.

$a = h_0/l_0$	Density $kg/mc$	Young's Modulus $kPa$	Poisson's ratio	Friction coefficient
1.5	2200	200	0.29	0.633

Table 2. Planar granular column collapse. Problem data.

MP particles per cell	Mesh size [m]	Time step [s]	Type of geometrical element
12	0.004	0.00001	Triangle

In Figure 2 the configurations at the dimensionless time  $T = t \sqrt{\frac{h_0}{l_0^3} \mathbf{g}}$  are shown. As one can observe, the results obtained by adopting an irreducible formulation (Figure 2(a)) are affected by volumetric locking in all the cases considered; the most visible effect of this issue is represented by the left and right upper corner, which doesn't disappear during the simulation (Figure 2(c)). On the other hand by using a mixed formulation (Figure 2(b)) the results look to be less affected by locking, mainly visible at  $T = 0.8$ , when the upper corners are completely disappeared.

In the presentation it is shown that by adopting a mixed formulation this issue can be fixed and the quality of the results can be improved, demonstrated by performing a qualitatively comparison of the configuration of the sample at different dimensionless time.

### 4. Conclusion

An implicit MPM is presented and used to solve problems, which involve granular flow, under the assumption of large displacements and large deformations. The code proposed is suitable to implement several constitutive laws; in this work a Drucker-Prager plastic model is used as first attempt to predict the behaviour of granular flow problem.

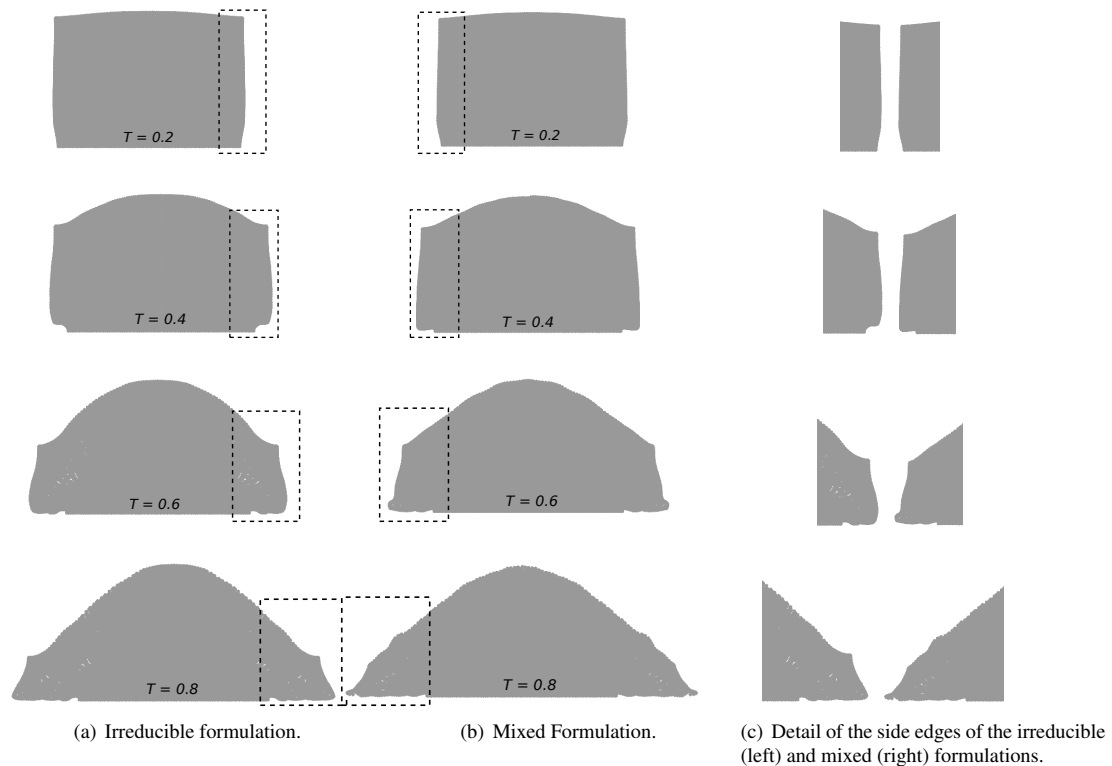


Fig. 2. Planar granular column collapse. Configuration of the deformed sample at different dimensionless time  $T$ . Comparison between the use of an irreducible formulation (Figure 2(a)) and a mixed one (Figure 2(b)); detail of the deformed sample side edges at different dimensionless time  $T$  in Figure 2(c).

Nevertheless, in the future further pressure-dependent constitutive laws will be considered and tested to improve the accuracy and quality of the numerical results.

A preliminary study is performed; a granular column collapse test case is considered and the results obtained by adopting an irreducible and a mixed formulation are compared. It is demonstrated that by using an irreducible formulation it is not possible to avoid the volumetric locking issue under the assumption of plastic incompressibility. In the presentation it is shown that the adoption of a mixed formulation is strictly recommended to avoid locking and to obtain more reliable results.

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